



## Thermal Properties of Normal and Light-Weight Aggregate Concrete with Elevated Temperature and Loading†

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This study was conducted for Ø100 × 200 mm specimens. The experiment was conducted at 20-700 °C in the state that the loads of 0, 20 and 40 % of compressive strength at room temperature were applied. The evaluation items included thermal strain, total strain and transient creep subjected to high temperature. The results of experiments showed that it was found that the transient creep by loading is constant regardless of the type of aggregate below 500 °C. However over 500 °C, the higher the thermal expansion coefficient of aggregate is, the transient creep by loading increases rapidly.

**Keywords:** Thermal strain, Crack, High temperature, Restraint force, Shrinkage strain.

### INTRODUCTION

In order to estimate the behaviour of concrete structures under high temperature, thermal properties of various materials such as aggregate, cement paste and admixture should be sufficiently reflected. Generally, the mechanical behaviours of concrete are described by theory of elasticity.

The concrete subjected to high temperature expands or contracts according to thermal properties of the components. In case of concrete when it is subjected to high temperature, it shows nonlinear behaviour by effects of spalling, reduction of compressive strength and elastic modulus and transient creep<sup>1-3</sup>. Effect of thermal properties of aggregate on concrete is very high. Therefore, CEN and CEB Codes suggest the models for performance-based fire safety design for normal weight aggregate concrete and light-weight aggregate concrete, considering thermal properties of aggregate<sup>4</sup>.

The aim of this study is to investigate the effect of high temperature and loading on thermal properties such as stress-strain curve, strain at peak stress and thermal strain. In addition, it aims to provide the actual data for performance-based fire safety design by analyzing the effect of thermal strain of aggregate and shrinkage strain by loading on mechanical properties of concrete.

### EXPERIMENTAL

The physical properties of materials used in this study are given in Table-1. The types of coarse aggregate included granite, ash-clay and clay aggregates.

The thermal expansion coefficients of coarse aggregate according to temperature are shown in Table-2.

**Concrete:** In this study, concretes are made by using granite, ash-clay and clay aggregate, respectively, which are

TABLE-1  
PHYSICAL PROPERTIES OF MATERIAL

Physical properties	PC	SF	Coarse aggregate		
			Granite	Ash-clay	Clay
Specific surface (cm <sup>2</sup> /g)	3,160	200,000	–	–	–
Specific gravity	3.1	2.2	2.67	1.68	1.79
Size (mm)	–	–	20.00	13.00	13.00
Absorption (%)	–	–	1.00	15.20	17.40

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TABLE-2 COEFFICIENT OF THERMAL EXPANSION ( $\times 10^{-6}/^{\circ}\text{C}$ )								
Aggregate type	Heating temperature ( $^{\circ}\text{C}$ )							
	20	100	200	300	400	500	600	700
Granite	6.1	4.8	7.4	9.8	12.2	14.5	21.1	21.2
Ash-clay	4.3	3.5	3.2	3.6	4.0	4.4	5.2	4.7
Clay	5.7	5.2	5.8	6.2	6.5	6.8	8.1	7.5

designated as GC, AC and CC, respectively. The mixing and fresh state of concrete are showed in Table-3. The concrete mixing was set to keep coarse aggregate at a constant volume. As for water to binder, it was set as 0.35 for GC and 0.33 for AC and CC, respectively. The light-weight concrete has low strength development compared to normal weight aggregate concrete.

TABLE-3 PROPORTION OF THE CONCRETE MIXTURES AND PROPERTIES OF THE FRESH CONCRETE			
Concrete type	GC	AC	CC
Water/cement	0.35	0.33	0.33
Water ( $\text{kg}/\text{m}^3$ )	165	155	155
Cement ( $\text{kg}/\text{m}^3$ )	470	432	432
Silica fume ( $\text{kg}/\text{m}^3$ )	-	38	38
Fine aggregate ( $\text{kg}/\text{m}^3$ )	692	687	687
Granite ( $\text{kg}/\text{m}^3$ )	1,075	-	-
Ash-clay ( $\text{kg}/\text{m}^3$ )	-	676	-
Clay ( $\text{kg}/\text{m}^3$ )	-	-	720
Unit weight ( $\text{kg}/\text{m}^3$ )	2,410	1,958	2,031
Slump (mm)	190	180	175
Air content (%)	3.3	3.5	3.6

**Preparation of specimen:** For evaluating mechanical properties at a high temperature, we manufactured  $\varnothing 100 \times 200$  mm specimens. The specimens were cured under water for 7 days, then air dry curing was conducted up to 180 days at a steady temperature and humidity chamber set as  $20 \pm 2^{\circ}\text{C}$ , R.H.  $50 \pm 5\%$ .

**Test methods:** An electric heating furnace was installed in the 2,000 kN capacity UTM loading machine so that loading and heating may be carried out simultaneously.

Fig. 1 shows heating curve of this study. The heating velocity was conducted at the temperature below  $1^{\circ}\text{C}$ , referring to previous studies and RILEM Code<sup>6,7</sup>. In order to heat concrete specimens at uniform temperature, the range from room temperature ( $20^{\circ}\text{C}$ ) to  $50^{\circ}\text{C}$  and the range from  $50^{\circ}\text{C}$  before reaching the target temperature were heated at  $0.77^{\circ}\text{C}/\text{min}$  and the other ranges were heated at the heating velocity of  $1^{\circ}\text{C}/\text{min}$ . In addition, it was set for the temperature difference between the inside and the outside to be within  $5^{\circ}\text{C}$  by keeping it at target temperature for 1 h.

In accordance with (Part 6-Thermal Strain) of RILEM TC 129-MHT, strain properties were measured and thermal behaviours of concrete at fire were analyzed<sup>6</sup>. The transient creep was calculated in accordance with (Part 7-Transient Creep) of RILEM TC 129-MHT<sup>7</sup>. The strain at peak stress was evaluated by the method presented in EN 1992-1-2:2004(E).

**RESULTS AND DISCUSSION**

**Strain of concrete:** Fig. 2 shows the thermal strain and total strain of concrete. Unstressed condition, for GC, the

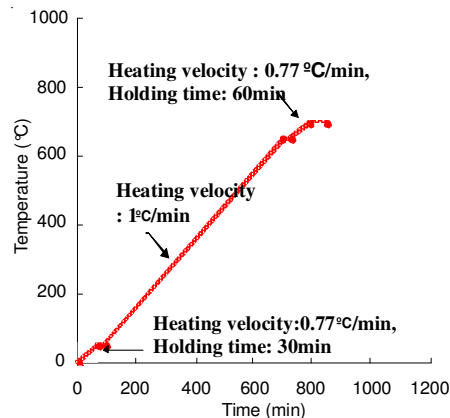


Fig. 1. Heating method

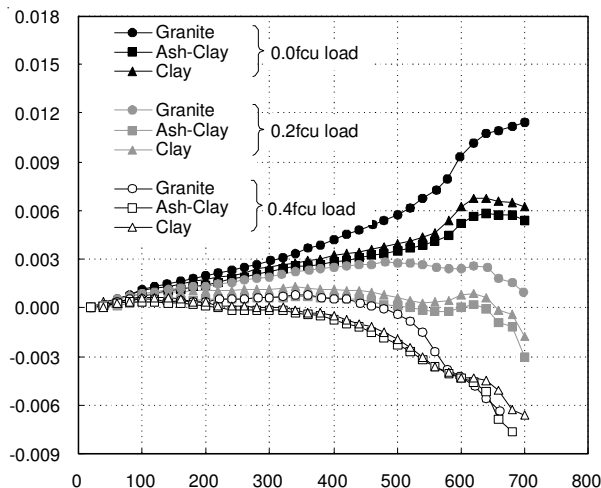


Fig. 2. Thermal strain and total strain

thermal expansion coefficient increased with elevated temperature. Ultimately, it showed the thermal expansion coefficient of 0.0012 at  $700^{\circ}\text{C}$ . GC shows the value between siliceous and calcareous aggregates.

As the thermal expansion coefficient is about a quarter compared to granite aggregate, in case light-weight aggregate is used to make concrete, it has smaller thermal strain than GC.

While, the thermal strain of AC at  $700^{\circ}\text{C}$  is 0.006, which is 50 % compared to GC and twice higher than 0.003, the thermal strain of aggregate. It is estimated as a phenomenon which occurs by increase of thermal stress due to formation of micro-structures as silica fume replaces 8 % of the cement weight in order to develop the prescribed strength.

Stressed condition, in case concrete is subjected to the loads of 0.2 and 0.4 fcu, the thermal strain of concrete is significantly controlled in general. GC was subjected to the load of 0.2 fcu, the strain continued to increase by  $300^{\circ}\text{C}$  and the strain convergence at 0.003 above  $300^{\circ}\text{C}$ . Under the load

condition of 0.4 fcu, a little increase of the strain or was kept by 400 °C, then it was rapidly shrunk above 500 °C and failed at 700 °C. In case of AC and CC, unlike GC, they kept the shapes of specimens before heating without thermal strain under the load condition of 0.2 fcu, whereas in case of being subjected to the load of 0.4 fcu, it was rapidly shrunk above 300 °C and the strain increased.

Fig. 3 shows the transient creep of concrete specimens to which loads and heat are applied. The transient creep constantly increased with temperature and it showed rapid shrinkage over 500 °C. Thus, it is analyzed that as for the transient creep, the effect of load is big below 500 °C, while the effect of temperature is dominating over 500 °C.

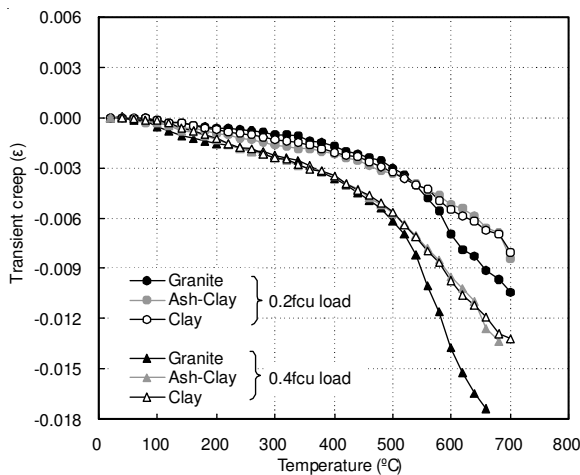


Fig. 3. Transient creep strain

For analyzing this, the thermal strain of concrete and the strain at maximum stress were compared and the results are shown in Figs. 4-6. As a result, it has been confirmed that the larger the residual strain of concrete is, the lower the residual ratio of strength is and the larger the thermal strain of aggregate is, the greater the effect is.

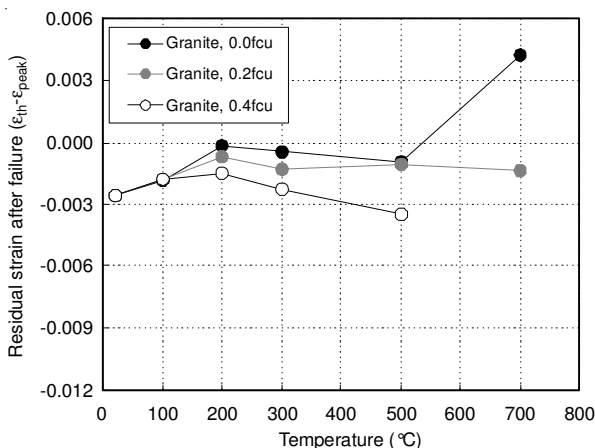


Fig. 4. Status of concrete after failure GC

**Conclusion**

With the increase in load, the concrete changes to shrinkage. The transient creep is small in case shrinkage strain by load is more influential than thermal expansion strain of aggregate. The transient creep is similar, regardless of the type

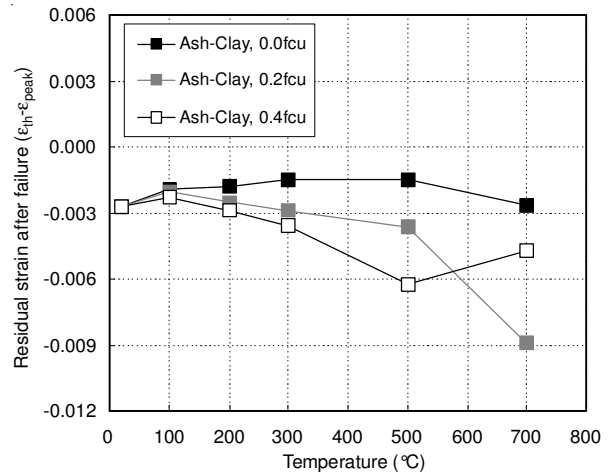


Fig. 5. Status of concrete after failure AC

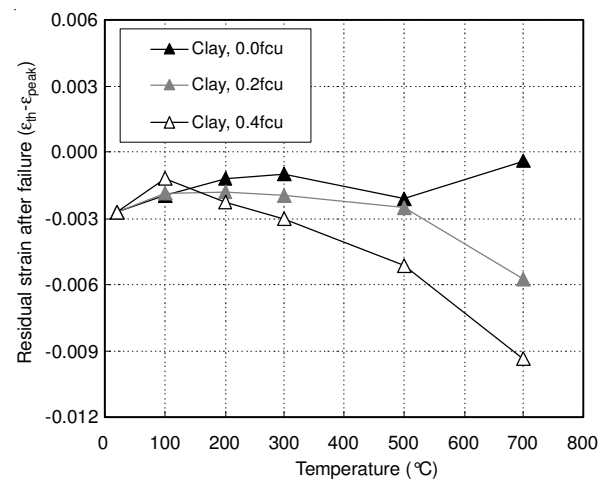


Fig. 6. Status of concrete after failure CC

of aggregate at the temperature below 500 °C. Over 500 °C, the concrete having high thermal strain of aggregate has rapid increase of transient creep along with the increase of load. If thermal strain of concrete is small and the load of 20-40 % is applied to it, the elastic deformation of concrete is kept, by which the transient creep is declined.

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