

Removal of Orthophosphate from Municipal Wastewater Using Chemical Precipitation Process in Ahvaz Wastewater Treatment Plant, Iran

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Phosphate removal such as orthophosphate, from municipal wastewater has become an environmental necessity, since the excessive phosphate ion content in municipal wastewater causes water eutrophication. According to environmental protection organization of Iran, maximum permissible concentration of residual phosphorus in treated municipal wastewater is 1 mg/L P. Almost all the phosphorus in natural water and wastewater is available in the form of phosphate. Phosphorus compounds in wastewater, after hydrolysis and biodegradation, change to solution orthophosphates. In this research, poly aluminium chloride and bentonite clay as nature coagulant were added and with low chemical cost, bentonite effect in improving poly aluminium chloride in municipal wastewater to primary and effluent of secondary clarifier and then effect of pH and dose of coagulant investigated on orthophosphate, COD, BOD and TSS in pH = 7 and optimal dose 15.55 mg/L polyaluminium chloride and 5.55 mg/L bentonite in influent wastewater to primary clarifier has been obtained 71.83, 46.1, 48.9 and 63.5 %, respectively. Under optimal condition, orthophosphate concentration was reached to 1 mg/L. So this compound coagulant, in lowest dose and consequently chemical cost decrease, was selected as suitable coagulant in removal of orthophosphate of effluent of treatment plant.

Key Words: Eutrophication, Orthophosphate removal, Wastewater advanced treatment.

INTRODUCTION

According to environmental protection organization of iran, maximum permissible concentration of residual phosphorus in treated municipal wastewater for discharge is announced to be 1 mg/L phosphorus¹. The adverse effects of eutrophication due to the presence of phosphate ions in water have been well documented. Phosphate ions discharged into water streams act as a major nutrient for aquatic life, which can lead to overgrowth of algae in lakes. This lowers the water quality by consumption of dissolved oxygen, resulting in the destruction of aquatic life². The average concentration of total phosphorus in raw municipal wastewater is 8 mg/L, about 10 % of it, is removed in primary sedimentation and the other 10-20 % in biological treatment, so the remainder 70 % is often discharged by secondary effluent³. Common forms of phosphorus in

wastewater are orthophosphate, polyphosphates and phosphates bonded to organic compounds. The last compounds release orthophosphate in aqueous solution too⁴. Nearly all the P-content of wastewater and natural waters may appear as phosphate⁵. It is obvious that advanced wastewater treatment facilities have to be employed to meet the discharge standard of phosphorus⁶⁻⁸.

Phosphorus removal can be accomplished either biologically or chemically⁹. Chemically,phosphate is most commonly removed by precipitation. Precipitation processes are capable of at least 90-95 % phosphorus removal at a reasonable cost¹⁰. Numerous substances have been used as coagulants, including aluminium and iron salts. In recent year, extensive researches have been conducted on coagulation process and various coagulants among polyaluminium chloride in literature. Polyaluminium chloride is a pre-polymerized coagulant is one of the most common coagulants in different water and wastewater treatment plants in countries such as USA, Canada, China, Italy, France and Britian¹¹. This compound forms multi-core complex in wet environment and this unique characteristic helps polyaluminium chloride work efficiency during coagulation process¹². In a polyaluminium chloride molecule, a large portion of aluminium appears in form of large oligomer polymers of aluminium¹³ cations with +7 ions a [Al₁₃(OH)₂₄O₄(H₂O)]¹⁷. So polyaluminium chloride, to reason the most production of positive charge in proportion to non-polymer coagulants in organic matter and colloid particles negative charge neutralization in wastewater have be the most effective¹³. Studies have been conducted to evaluate the potential of clay minerals like bentonite in the removal of phosphorus. The abundance of bentonite and it is low cost are likely to make it a strong candidate as an adsorbent for the removal of phosphorus from wastewater^{14,15}.

Zouboulis and Tzoupanos¹⁶ reported that the phosphorus removal efficiency in wastewater, in dose 30-60 mg/L polyaluminium chloride has been obtained 75-98 %. Wolf and Lind¹⁷, using bentonite clay, in dose 200 mg/L, 75 % solution phosphorus after 1 h of settling time, removed of phosphorus.

In general, the degree of phosphorus removal by chemical precipitation is affected by many factors, such as pH, alkalinity, coagulant dose and type, pollutants concentration in wastewater, speed of flash mixing and other interfering substances¹⁸. So in this study, investigated removal of orthophosphate from municipal wastewater in Ahvaz west wastewater treatment plant using chemical precipitation process by compound of polyaluminium chloride and bentonite clay.

EXPERIMENTAL

This research is a bench scale experimental type study. Influent wastewater to primary and effluent of secondary clarifier was collected from municipal wastewater treatment plant in Ahvaz city, Iran. The characteristics of wastewater are given in Table-1.

TABLE-1 PHYSICO-CHEMICAL CHARACTERTISTIC OF MUNICIPAL WASTEWATER						
Parameters	Influent wastewater	Effluent of				
	to primary clarifier	secondary clarifier				
pH	7	7				
TP (mg/L)	5	3				
Orthophosphate (mg/L)	2.9-3.1	2.7-2.85				
COD (mg/L)	279.68	71.33				
BOD (mg/L)	142.67	35.65				
TSS (mg/L)	320	46				

The coagulation experiments were carried out at the laboratory temperature (25 °C) using a jar test (model JLT6) with a 6 paddle stirrer. In each of the tests, 1000 mL of sample was taken in the jar. The pH was adjusted to desirable level with the addition of alkali (1 N NaOH) or acid (0.1 N HCL). The coagulant was added under stirring. Rapid mix took place for 2 min, at a speed of 120 rpm, followed by slow mix for 10 min, at 40 rpm. The settling period lasted for 1 h. After the settling period, samples were taken and analyzed immediately for orthophosphate and sludge volume index (SVI) in all of

the sample and COD, BOD and TSS in optimal condition of coagulant. At the end of each stage, the effect of each parameter on orthophosphate removal was determined using covariance analyzes and related graphs were depicted in an MS Excel sheet and (p < 0.05) was significant.

In this study, the cyberscan pH 310 digital pH meter (EUTECH company) was used and digital balance with Sartorius model with accuracy 0.0001 g for weight of the coagulant. Orthophosphate by ascorbic acid method to number 4500 PE and COD by titration method to number 5220 C using of DR/ 5000 spectrophotometer, SVI to 2710 D method, BOD to BOD5 method to number 5210 B and TSS to weight method in standard methods for the examination of water and wastewater book (2005) was determined¹⁹.

RESULTS AND DISCUSSION

In this study, removal efficiency of orthophosphate by compound of polyaluminium chloride and bentonite clay was studied under various pH conditions. Figs. 1 and 2 shows the effect of pH on orthophosphate removal from influent wastewater to primary and effluent of secondary clarifier sing compound of polyaluminium chloride and bentonite clay.



Influent wastewater to primary clarifier

Fig. 1. Effect of pH on removal of orthophosphate concentration by addition of polyaluminium chloride and bentonite in every one of sampling points

Fig. 2 indicates that with increase of pH, orthophosphate removal efficiency increases. The most efficiency of orthophosphate removal in pH = 10 in influent wastewater to primary and effluent to secondary clarifier respectively equal 89.67 and 78.45 % was obtained. But due to the main aim in this test, orthophosphate concentration decrease to 1 mg/L (standard phosphorus in treatment plant effluent), so pH = 7 with orthophosphate concentration low of 1 mg/L in different points of sampling was selected as optimal pH that non-requirment to pH adjustment and change in wastewater natural conditions.



Influent wastewater to primary clarifier ____Effluent of secondary clarifier



RESULT OF THE DETERMINATION OF OPTIMUM DOSE OF COMPOUND COAGULANT IN INFLUENT WASTEWATER TO PRIMARY CLARIFIER IN ORTHOPHOSPHATE REMOVAL							
Coagulants	Coagulant dose	Average of influent	Average of effluent	Orthophosphate removal	SVI		
ratio	PAC: Bentonite (mg/L)	orthophosphate (mg/L)	orthophosphate (mg/L)	efficiency (%)	(mL/g)		
1:1	10:12.5	3	1.365	54.50	36.0		
1:2	13.33:8.3	3	1.19	60.33	40.0		
1:3	15:6.25	3	1.02	66.00	45.0		
1:3.5	15.55:5.55	3	0.845	71.83	50.0		
1:4	16:5	3	0.731	75.61	51.7		
1:4.5	16.36:4.54	3	0.716	76.116	53.0		
1:5	16.66:4.16	3	0.702	76.60	54.0		
1:6	17.14:3.57	3	0.656	78.133	56.0		
7:1	17.5:3.125	3	0.596	80.133	59.0		
DAC Delectronicity allocide)							

TABLE-2

PAC = Poly(aluminium chloride)

TABLE-3 RESULT OF THE DETERMINATION OF OPTIMUM DOSE OF COMPOUND COAGULANT IN EFFLUENT OF SECONDARY CLARIFIER IN ORTHOPHOSPHATE REMOVAL

Coagulants ratio	Coagulant dose PAC: Bentonite (mg/L)	Average of influent orthophosphate (mg/L)	Average of effluent orthophosphate (mg/L)	Orthophosphate removal efficiency (%)	SVI (mL/g)
1:1	15:12.5	2.8	1.14	59.30	41.0
1:1.5	18:10	2.8	1.10	60.714	43.0
1:2	20:8.33	2.8	0.936	66.553	49.2
1:2.5	21.43:7.143	2.8	0.83	70.36	56.0
1:3	22.5:6.25	2.8	0.75	73.214	57.0
1:4	24:50	2.8	0.60	78.571	59.0
1:5	25:4.166	2.8	0.522	81.34	61.0
1:6	25.71:3.57	2.8	0.445	84.107	63.0
7:1	26.25:3.125	2.8	0.35	87.50	65.0

Optimum pH of polyaluminium chloride in work of Zouboulis and Tzoupanos²⁰ was range of 6.5-8.5 in phosphorus removal in municipal wastewater in dose of 40 and 50 mg/L polyaluminium chloride, phosphorus concentration decrease to 0.1 and 0 mg/L respectively. Cucurella and Renman²¹, after using of ratio of bentonite clay to solution equal 1/20 and in pH = 4-12, phosphorus absorption capacity in pH equal 7.5, 8.5 and 9 obtains respectively 3.5, 4.3 and 9.5 gp/kgclay as the most absorption capacity²¹, then the coagulants in certain pH in wastewater have be the most solubility and hydrolysis of them in wastewater rapidly and form hydrolysis productions that are metal ion with positive charge or metal hydroxids (hydrolysis productions depends to coagulant optimum pH). So phosphate ion in wastewater was absorbed to metal ions with positive charge or metal hydroxides and after of the phosphate metal hydroxide formation, is settle and precipitation happen.

The influence of compound of polyaluminium chloride and bentonite dosage on orthophosphate removal during the coagulation in influent wastewater to primary and effluent of secondary clarifier was shown in Tables 2 and 3. From the experimental data it is observed that the residual concentration of orthophosphate ions decrease with the increases in quantity of compound coagulant. The most efficiency of orthophosphate removal with confidence of 95 % obtain in dose 17.5 mg/L polyaluminium chloride and 3.12 mg/L bentonite in influent wastewater to primary clarifier and dose 26.35 mg/L polyaluminium chloride and 3.12 mg/L bentonite in effluent of secondary clarifier respectively reach to 80.13 and 87.5 %. Regarding the results of covariance test (p < 0.05) a significant difference can be found between coagulants dose and orthophosphate removal percent. Due to main aim in this tests, orthophosphate concentration decrease to 1 mg/L, dose 15.55 mg/L polyaluminium chloride and 5.55 mg/L bentonite in influent wastewater to primary clarifier and 20 mg/L polyaluminium chloride and 8.33 mg/L bentonite in effluent of secondary clarifier with final orthophosphate concentration and orthophosphate removal efficiency respectively in every one of the sampling points equal (0.845 and 71.83 %) and (0.94 and 66.55 %), as optimum dose in this test was selected. Optimum dose of polyaluminium chloride in findings of Zouboulis and Tzoupanos¹⁶, was range of 50-60 mg/L that in this dose, phosphate concentration of 23 mg/L, decrease to low of 1 mg/L. According to Wolf and Lind¹⁷, phosphorus removal per cent in dose 200 mg/L bentonite and after of 1 and 2 h of settling time respectively equal 75 and 84 % were obtained. Then determination of optimum conditions of coagulants in wastewater, depended to quality and quantity characterize of wastewater and chemical property that due to this factors, coagulants in certain range of dose, show the most removal of phosphorus.

The produced sludge in wastewater physical-chemical treatment is due to reaction between organic material, suspended solids and another contaminats in wastewater with productions of coagulant hydrolysis in wastewater. Tables 2 and 3 showed that with increase of coagulants dose, sludge volume index increase. In other words, it is noted that with increase of coagulants dose, form the coarse flocs, with the most solidity and high settling rate that to cause increase of produced sludge volume and consequently increase of sludge volume index.

COD, BOD and TSS removal efficiency in every one of sampling points in optimum conditions of compound coagulant respectively equal (46.1, 48.9 and 63.5 %) and (48.3, 55.5 and 65.1 %) obtain. Since coagulants performance in wastewater depened to different factors such as pH of wastewater, phosphorus concentration, organic matter, suspended solids and another contaminant in wastewater, so with increase of contaminants in wastewater, removal efficiency by coagulants decreases. In this test, observes in influent wastewater to primary clarifier, to reason of high concentration of TSS, COD and BOD and in the low dose of coagulant, removal efficiency of contaminants in proportion to effluent of secondary clarifier is low. Amuda and Amoo²², with compound of 25 mg/L polyelectrolyte and 100 mg/L ferric chloride, observed TSS, COD and phosphorus removal respectively reach to 99, 91 and 97 % that reason to has been the highest dose of coagulants for this contaminant removal. Due to aim in this research, was the orthophosphate removal by compound coagulant, so in optimum conditions of coagulant, has been investigated the suspended solids and organic matter removal and obtain the lowest removal efficiency of COD, BOD and TSS.

Conclusion

The work confirms, the optimal conditions for the process of orthophosphate removal from municipal wastewater by treating it with compound of polyaluminium chloride and bentonite clay. The experimental data show that orthophosphate separation is a complex physicochemical process. Some authors believe that the chemical processes play a fundamental role and that elimination of phosphorus is the result of AlPO₄ precipitation, which can be complicated by the simultaneous precipitation of Al(OH)₃. These hydroxides perform a flocculant function, facilitating the separation of the precipitated orthophosphate. Other authors believe that the phenomena of flocculation and hydroxide formation are responsible for orthophosphate ion removal by absorption of the orthophosphate ions. Based on the studies conducted²³, we believe that the two type of phenomena work simultaneously.

Orthophosphate removal efficiency using coagulants, depends to coagulation conditions, wastewater quality, treatment process and chemical property.

Influent point to primary clarifier, to reason the less dose of compound coagulants for orthophosphate removal and decrease of chemical cost in proportion to effluent point of secondary clarifier, was selected as the optimum sampling point. Also removal efficiency of COD, BOD and TSS by compound coagulant and in this sampling point respectively reach to 46.1, 48.9 and 63.5 %. Since influent wastewater to primary clarifier should not containing the low concentration of organic material such as COD and BOD for wastewater biological treatment in aeration chamber, so the highest removal efficiency of this contaminants, before of biological treatment, disorder in wastewater biological treatment process. Due to removal efficiency of organic matter in this tests, so chemical precipitation process can not be disorder in wastewater biological treatment process. In effluent point of secondary clarifier, to reason the highest dose of coagulant and increase of the chemical costs and non-settling chamber in Ahvaz west wastewater treatment plant, so establishment of mixing, flocculation and settling chamber, cause to increase of the costs in wastewater treatment process units, so selection of this point as the optimum sampling point, is not economical.

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